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"Using vegetation properties to predict flow resistance and erosion rates"

Nick Kouwen University of Waterloo Waterloo, Canada

US Dept. of Agriculture, Stillwater Okla. Lab. 1985 Test channels 5% slope (This run 1.7 m³/s 60 cfs)

Measurement of soil profile and count of grass density

View through side after approximately 5 day run

UW Lab – plastic model of grass Circa 1969

Use of pitot tube as reference to measure height



Click on insert to view movie



Dimensional Analysis (List of everything by Fenzl):

 $\psi(\rho,\mu,\sigma,h,b,J,k,\lambda,\lambda_1,\lambda_2,\ldots,\lambda_n,\theta_p,\theta_s,\beta,V,y_n,g,S_f) = 0$

After applying the Buckingham Π theorem (by NK)

$$\mathbf{f} = \emptyset \left\{ \frac{k}{y_n}, \frac{k}{h}, \frac{h}{\left(\frac{MEI}{\tau_o}\right)^{\frac{1}{4}}} \right\}$$

Only the most important variables are retained

M =stem density =
$$\frac{D^2}{\lambda_1 \lambda_2}$$

- f =D W friction factor
- y_n =depth of flow
- k =roughness height in flow
- h =vegetation length
- τ_o = shear stress = γ ALS

(NOTE: $k \le h$ and $k \le y_n$)



From Experiments:



So:



Kouwen, Unny & Hill, ASCE J. of the Hydraulics Division, May, 1973, 713-728





i.e. f = fn (relative roughness, smoothness)

In practical terms, this means:

$$\frac{1}{\sqrt{f}} = a + b \log_{10}\left(\frac{y_n}{k}\right)$$

where a and b are coefficients that vary with the value of k/h - or smoothness (re: Keulegan, 1938)

$$a = f_1\left(\frac{k}{h}\right)$$
 $b = f_2\left(\frac{k}{h}\right)$

$$V = \sqrt{\frac{8g}{f}} \sqrt{y_n S}$$

$$V = \sqrt{8g} \left[a + b \log \left\{ y_n \middle/ 0.14h \left\{ \frac{\left(\frac{MEI}{\gamma y_n S} \right)^{0.25}}{h} \right\}^{1.59} \right\} \right] \sqrt{y_n S}$$

All we now need is MEI for natural vegetation

So how do we get values for MEI for natural vegetation?



Field methods to obtain MEI



Kouwen, IAHR J. Hydraulic Research, 1988, Vol. 26, 559-568



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MEI can also be based on the vegetation height:





A comparison of the two methods of Estimating MEI yields similar results.

Kouwen, IAHR J. Hydraulic Research, 1988, Vol. 26, 559-568





Low soil shear

High soil shear

As long as y' has some value, there is little shear at the bed and channel is stable





Scour hole developed in 250 mm long Bermuda Grass after 5 day run Slope 5% Flow 1.7 m³/s

k/h for this case was ~ 0.35

Erosion Model:

$$\tau_{e} = \rho g y_{n} S \left[(1 - C_{F}) (\frac{n_{s}}{n})^{2} \right]$$

 $Er = kk(\tau_e - \tau_c)^{a'}$

Effective shear (Temple)

Duboys sediment transport equation to estimate detachment













Conclude:

Onset of instability can be predicted by Temple-Duboys model

kk and **a** in Duboys eqn. the same for all plots Only the Cover Factor C_f adjusted

Details: Samani & Kouwen. ASCE J. Hyd. Eng. Jan. 2002

Drag on trees

Straight, level road with no cross-wind
10 – 80 km/hr in 10 km/hr increments
Cedar, Spruce, Austrian and White pine trees
Up to 3.5 m in height. Freshly cut.
(please note: in Canada we normally drive on the right)



Dense cedar saplings in a flume with one tree in the force balance Saplings of cedar, spruce and two species of Pine were tested in water and air.

Drag was measured and recorded for a period of time using force balance tables and a load cell.

In water, the additive principle was tested. Different patterns were used.

Fathi-Moghadam & Kouwen ASCE J. Hydraulic Eng. Jan. 1997 pp. 51-57

In water:

In air:

$$C_{d} = fn_{1} \left(A_{o}, V, \rho, y_{n}, J, g, \mu, h, \xi, \phi, \omega, l_{1}, ..., l_{n}\right)$$

$$fn_{2} \left(C_{d}, \xi, \phi, \frac{A_{o}}{y_{n}^{2}}, \frac{l_{1}}{y_{n}}, \frac{l_{2}}{y_{n}}, \frac{h}{y_{n}}, \frac{\rho V^{2} y_{n}^{4}}{J}, \frac{\rho V y_{n}}{\mu}, \frac{V^{2}}{g y_{n}}, \frac{\omega y_{n}}{V}\right) = 0$$

$$fn_{3} \left(C_{d} \left(\frac{A}{\forall}\right)h, \frac{\rho V^{2}h^{4}}{\xi J}, \frac{\rho V y_{n}}{\mu}, \frac{V^{2}}{g y_{n}}, \frac{\omega y_{n}}{V}\right) = 0$$

$$C_{d} \left(\frac{A}{\forall}\right)h = fn_{4} \left(\frac{\rho V^{2}h^{4}}{\xi J}\right)$$

$$f = 4C_{d} \left(\frac{A}{a}\right) \qquad f = \alpha \left(\frac{V}{\sqrt{\frac{\xi E}{\rho}}}\right)^{\beta} \qquad \xi E = Nf_{1}^{2} \left(\frac{m_{s}}{h}\right)$$

$$f = 4C_d \left(\frac{A}{a}\right)$$



 $\xi E = N f_1^2 \left(\frac{m_s}{h} \right)$

(Nickas & Moon, 1988)

A = Momentum Absorbing Area

- a = horizontal area of canopy
- ξ = parameter to account for plant deformation due to flow velocity

 ξE = vegetation index

 Nf_1 = natural frequency of the tree

 $m_s = total tree mass$

h = height of the tree (or canopy)

Kouwen & Fathi-Moghadam, ASCE J. Hydraulic Eng., Oct. 2000, 732-740



Shaker table and spectrum analyzer to Obtain natural frequency of the tree.

Trolley and load cell to measure Force on a tree Accelerometer to obtain Natural Frequency







Air and Water Tests



Summary

- Dimensional analysis provides a unified approach to flow resistance due to vegetation.
- The flexural characteristics of vegetation have a major effect on resistance.
- The biomechanical properties of grass as they affect flow resistance are in essence an "equivalent to plastic stiffness".

Thank you

Presentation at http://www.watflood.ca