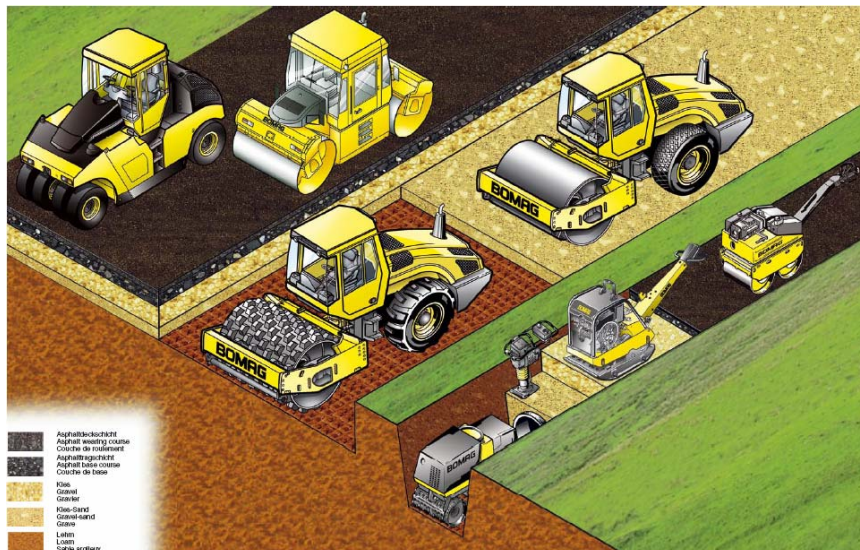
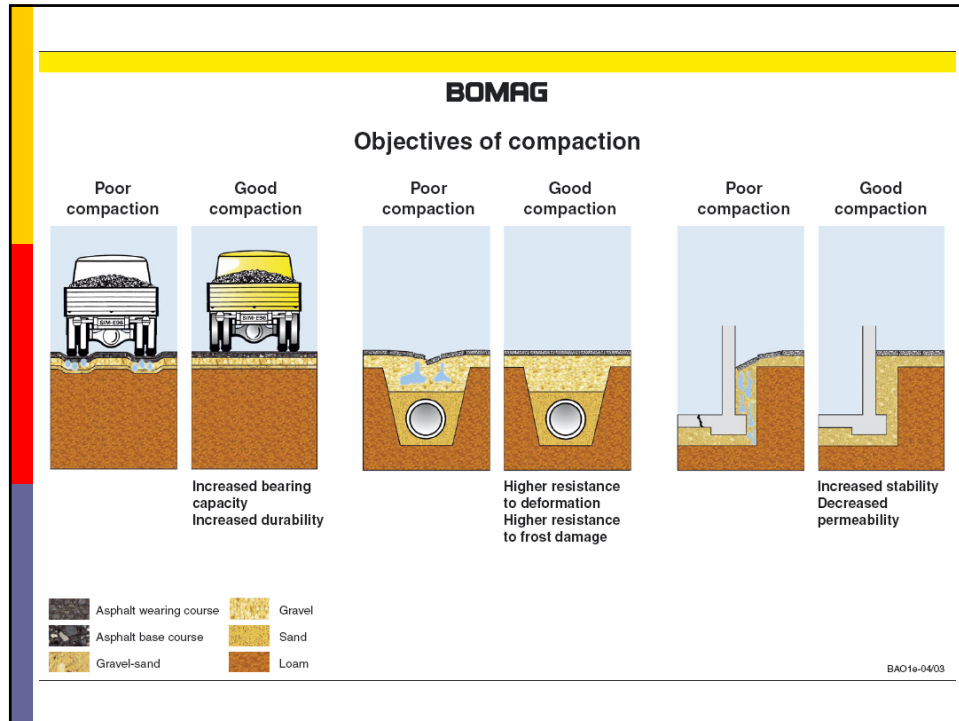


Compaction

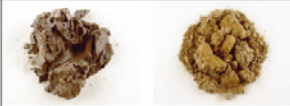
Asphalt, Roads, Trench Restoration





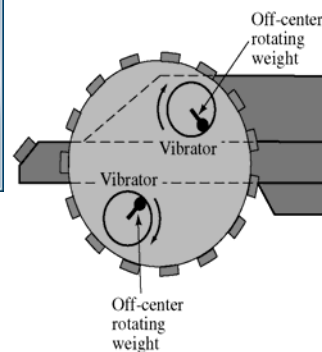
Goal of Compaction

- Place loose soil in the field and compact it to make soil strong as possible
 - Maximum shear strength
 - Very little settlement
- Find soil lowest $e_{\min}$ highest dry unit weight

BOMAG Soil compaction					
Fine aggregate		Coarse aggregate		Rockfill	
Clay	Silt	Sand	Gravel	Cobbles	Boulders
Application primarily for • dam construction • embankments for road, rail track and airport construction • trenches and backfills • sanitary landfill construction		Application primarily for • embankments subbases and bases for road, rail track and airport construction • foundations for buildings • trenches and backfills		Application primarily for • dam construction • embankments for road, rail track and airport construction • foundations for buildings	
Clay < 0,002mm Silt 0,002-0,06mm		Sand 0,06 - 2 mm Gravel 2 - 60 mm		Cobbles > 60 mm Boulders > 100 mm	
					
Compactability • difficult to compact due to cohesion • compaction effect depends strongly on water content • material needs high compaction energy		Compactability • depends on grading • too much compaction may be detrimental		Compactability • layer thickness should be three times thicker than max. particle size • material needs high compaction energy	
Compaction equipment primarily • heavy and medium size single drum rollers (smooth and padfoot drum) • trench compactors and heavy plates		Compaction equipment • vibratory tandem rollers and single drum rollers (smooth drum) • heavy and medium size plates		Compaction equipment primarily • heavy single drum rollers • heavy plates	

9A01a-10/03

Field Compaction Equipment



Vibratory Smooth Wheel Rollers

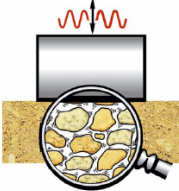
Figure 4.12 Principles of vibratory rollers

BOMAG

Vibratory compaction single drum rollers with circular exciter system



low
amplitude



Compaction principle
static pressure and
dynamic energy



high
amplitude

Application

- thin layers
- granular materials
- granular bases
- overlay compaction on thick granular layers

Key parameters

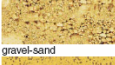
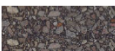
- static linear load
- vibrating mass
- amplitude
- frequency

Application

- thick layers
- granular and cohesive materials
- cement bound materials
- subbases
- embankments
- dams

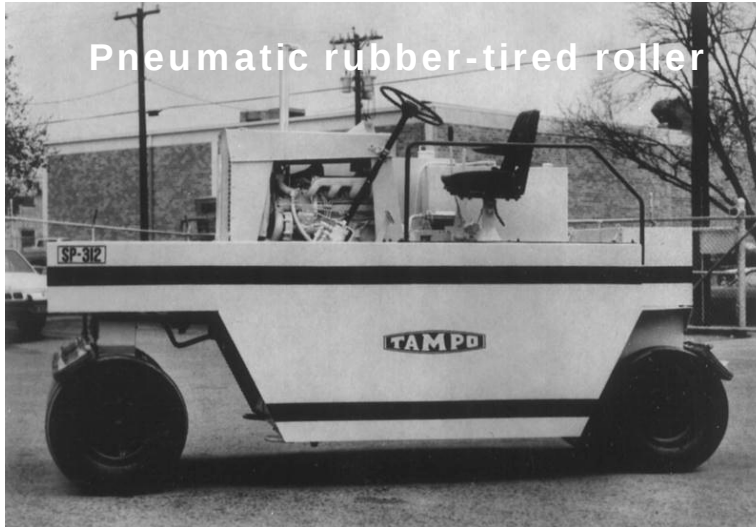
 Gravel-sand

BA01a-17/04

BOMAG						
Application guidelines - vibratory rollers						
application		roller	static linear pressure kg/cm	amplitude mm	frequency Hz	rolling speed km/h
earthworks dams			≥ 30	≥ 1,5	28 - 35	1 - 2,5
						
granular bases subbases			≥ 10	≥ 0,4	28 - 60	2 - 4
						
						
asphalt base course			10 - 30	0,35 - 0,9	30 - 60	2 - 4
asphalt wearing course				≤ 0,5	40 - 60	2 - 6

Field Compaction Equipment

Pneumatic rubber-tired roller

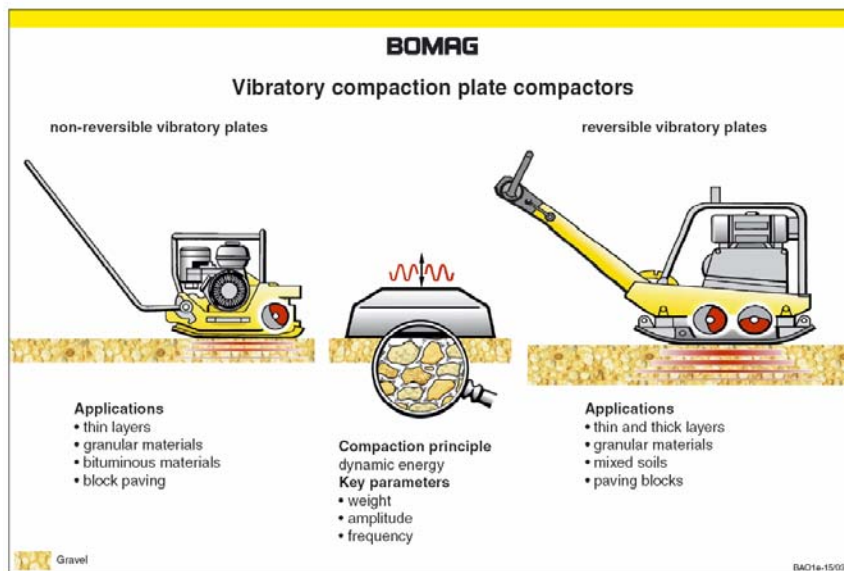


Field Compaction Equipment

Vibratory sheepfoot roller

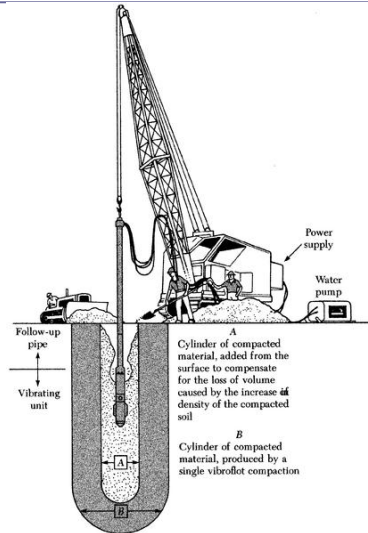


Soil Compaction Equipment



Field Compaction Equipment

- ▣ Vibroflotation unit
- ▣ Vibrate loose sand deposits

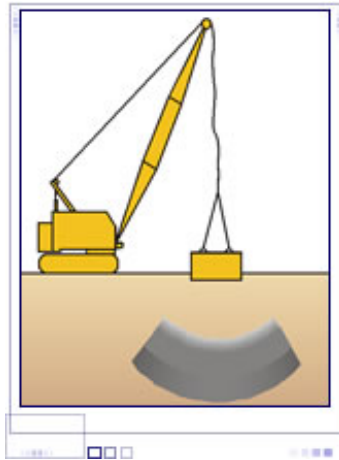


Field Compaction Equipment

- ▣ Drop weight 10 to 20 feet at close centres
- ▣ Makes craters
- ▣ Loss fill and sand

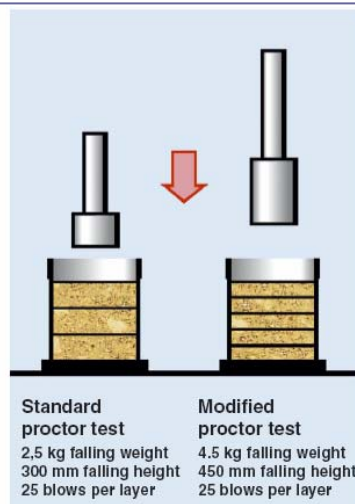


Field Compaction Equipment

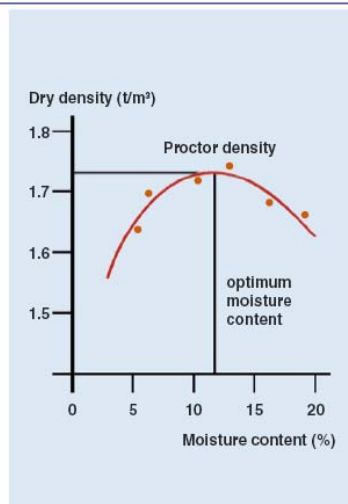


General Soil Type	Most Likely Fill Class	Most Likely AASHTO Soil Type	Degree of Saturation	Suitability for D.D.C.
Pervious deposit in the grain size range of boulders to sand with 0% passing the #200 sieve Coarse portion of Zone 1*	Building Rubble Boulders Broken Concrete	A-1-a A-1-b A-3	High or Low	Excellent
Pervious deposits containing not more than 35% silt Fine portion of Zone 1*	Decomposed Landfills	A-1-6 A-2-4 A-2-5	High Low	Good Excellent
Semi-Pervious soil deposits, generally silty soils containing some sand but less than 25% clay with P1 < 8 Zone 3*	Clay Fill Mine Spoil	A-6 A-7-5 A-7-6 A-2-6	High Low	Not Recommended Fair-minor improvements. Water content should be less than plastic limit
Miscellaneous fill including paper, organic deposits, metal and wood	Recent Municipal Landfill	None	Low	Fair-long term settlement anticipated due to decomposition. Limit use to embankments
Highly organic deposits peat-organic silts		None	High	Not recommended unless sufficient granular fill added and energy applied to mix granular with organic

Proctor Test



Proctor test



Proctor curve

Standard Proctor Test

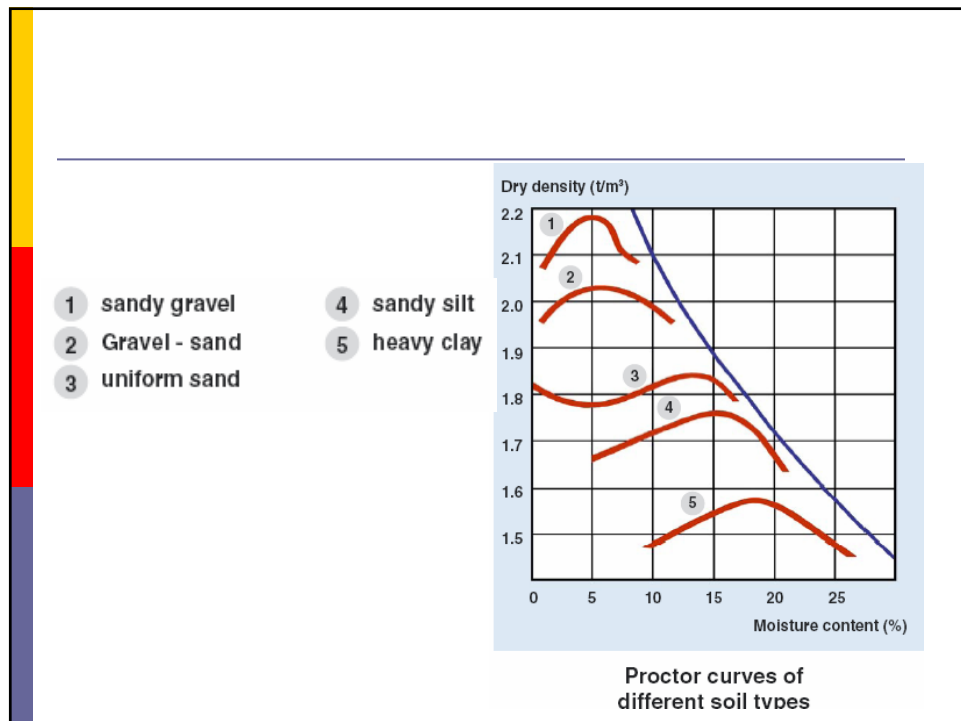
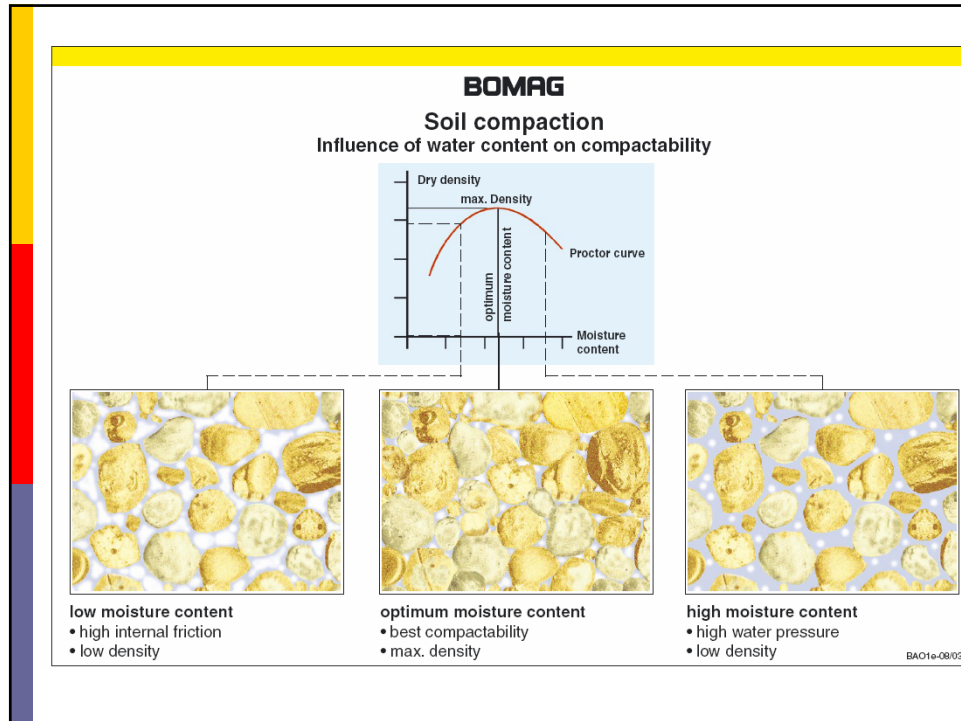
Table 4.2 Specifications for standard Proctor test (Based on ASTM Test Designation 698)

Item	Method A	Method B	Method C
Diameter of mold	101.6 mm	101.6 mm	152.4 mm
Volume of mold	943.3 cm ³	943.3 cm ³	2124 cm ³
Weight of hammer	24.4 N	24.4 N	24.4 N
Height of hammer drop	304.8 mm	304.8 mm	304.8 mm
Number of hammer blows per layer of soil	25	25	56
Number of layers of compaction	3	3	3
Energy of compaction	591.3 kN-m/m ³	591.3 kN-m/m ³	591.3 kN-m/m ³
Soil to be used	Portion passing No. 4 (4.75 mm) sieve. May be used if 20% or less by weight of material is retained on No. 4 sieve.	Portion passing 9.5-mm sieve. May be used if soil retained on No. 4 sieve is more than 20%, and 20% or less by weight is retained on 9.5-mm sieve.	Portion passing 19-mm sieve. May be used if more than 20% by weight of material is retained on 9.5-mm sieve, and less than 30% by weight is retained on 19-mm sieve.

Modified Proctor Test

Table 4.3 Specifications for modified Proctor test (Based on ASTM Test Designation 1557)

Item	Method A	Method B	Method C
Diameter of mold	101.6 mm	101.6 mm	152.4 mm
Volume of mold	943.3 cm ³	943.3 cm ³	2124 cm ³
Weight of hammer	44.5 N	44.5 N	44.5 N
Height of hammer drop	457.2 mm	457.2 mm	457.2 mm
Number of hammer blows per layer of soil	25	25	56
Number of layers of compaction	5	5	5
Energy of compaction	2696 kN-m/m ³	2696 kN-m/m ³	2696 kN-m/m ³
Soil to be used	Portion passing No. 4 (4.75 mm) sieve. May be used if 20% or less by weight of material is retained on No. 4 sieve.	Portion passing 9.5-mm sieve. May be used if soil retained on No. 4 sieve is more than 20%, and 20% or less by weight is retained on 9.5-mm sieve.	Portion passing 19-mm sieve. May be used if more than 20% by weight of material is retained on 9.5-mm sieve, and less than 30% by weight is retained on 19-mm sieve.



Impact of No. Blows

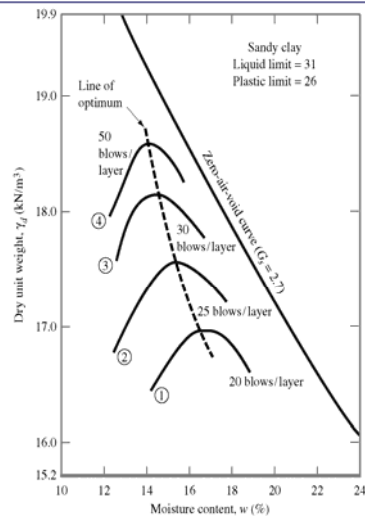


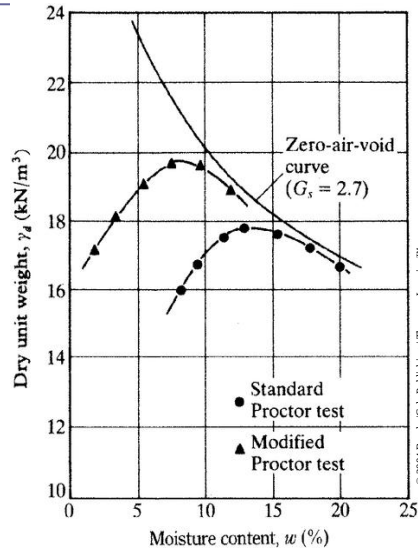
Figure 4.6 Effect of compaction energy on the compaction of a sandy clay

Impact of No. Blows

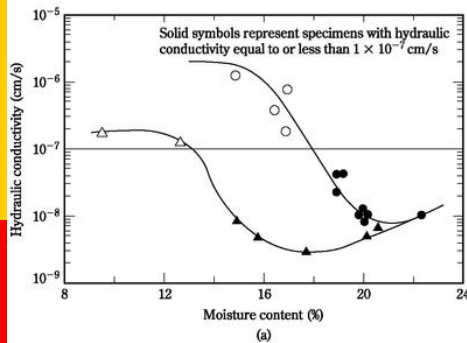
Table 4.1 Compaction energy for tests shown in Figure 4.6

Curve number in Figure 4.6	Number of blows/layer	Compaction energy (kN-m/m ³)
1	20	473.0
2	25	591.3
3	30	709.6
4	50	1182.6

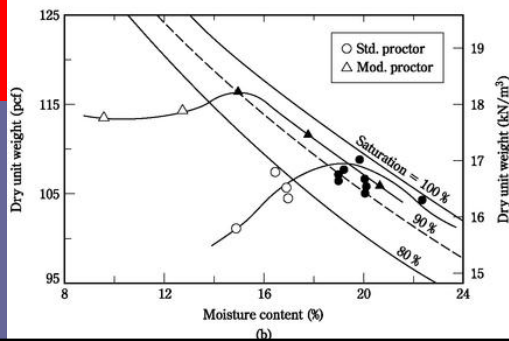
Impact of Proctor Method



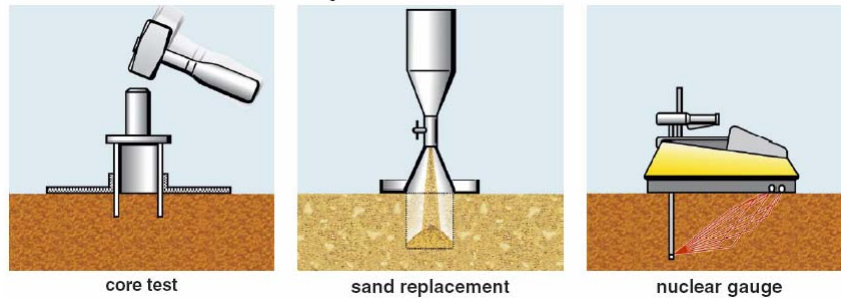
- Increase energy
- increase dry unit weight
- Decrease optimum moisture content



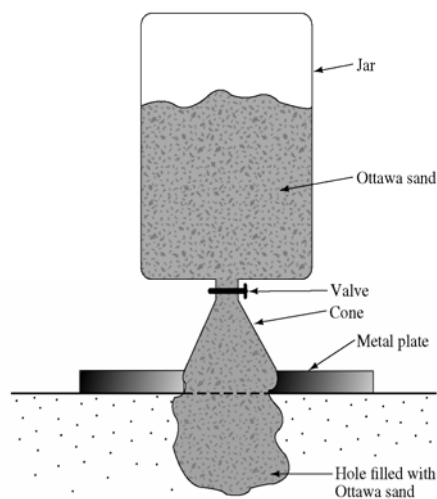
- Influence of compaction on soil hydraulic conductivity



Soil Compaction Measurement



Soil Compaction Measurement



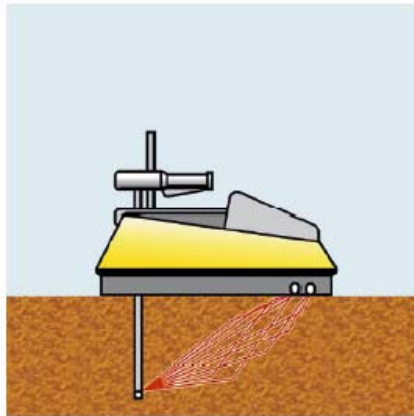
Use dry sand with known dry density and specific gravity

Use dry sand to get volume of hole

Quick and reliable method

Figure 4.15 Field unit weight by sand cone method

Soil Compaction Measurement



nuclear gauge

- Use radioactive material to get moisture content and soil density
- Quick method
- Reliable if calibrated
- Radioactive device therefore special transportation and rules must be followed

Compaction Specification

$$\text{Compaction level (\%)} = \frac{\text{Insitu dry unit weight}}{\text{Max. dry unit weight (Proctor)}} \times 100$$

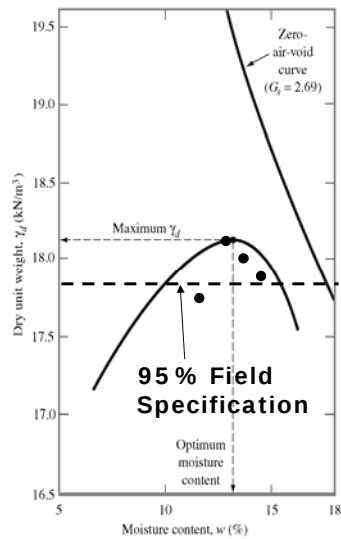
Standard Proctor Specification

- 95 to 100 percent of MDUW

Modified Proctor Specification

- 92 to 98 percent of MDUW

Compaction Specification



- Make sure compacted soil same as Proctor material (grain size distribution analysis)
- Add water to soil if too dry